



Stormwater Treatment Measure Sizing and Design Considerations

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Presentation Overview

- Sizing/Design of Self-Treating and Self-Retaining Areas
- Sizing/Design of Treatment Measures
 - Determining the Water Quality Design Flow and Volume
 - Bioretention and Flow-Through Planters
 - Pervious Pavement and Infiltration Trenches
 - High-Rate Media Filters

Site Design Measures to Reduce Runoff Requiring Treatment

- Self-Treating Areas
- Interceptor Tree Credits
- Self-Retaining Areas



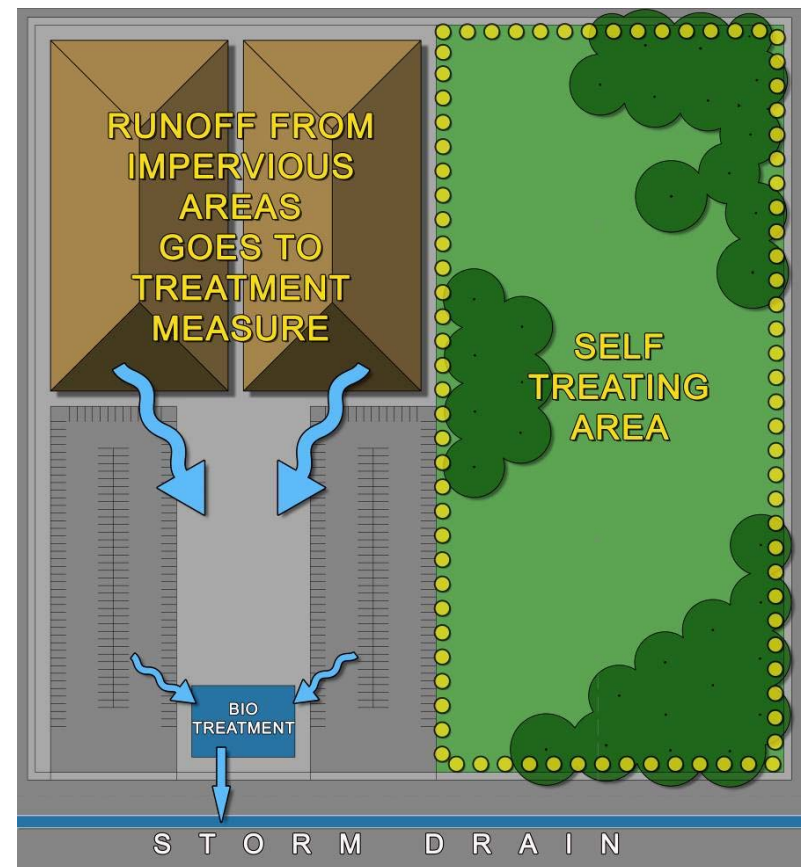
Self-Treating Area

- Pervious area that treats rain falling on itself only, via ponding, infiltration and ET
 - Landscaping
 - Green roof
 - Pervious pavement
 - Artificial turf
- Landscaped areas must retain approx. 1" of rain
- Pervious pavement and artificial turf must be designed to store and infiltrate the C.3.d amount of runoff in order to qualify as self-treating areas



Self-Treating Areas Reduce the Area Requiring Treatment

- Runoff from **pervious** portions of the project (after infiltrating 1") can flow directly to the storm drain (if not mixing with runoff from impervious areas)
- Runoff from **impervious** areas can flow to a smaller treatment measure





“Interceptor” Tree Credits

- Self-treating area credit allowed based on the interception of rainwater by the tree canopy
- Intended for small areas that can’t be treated

Type of Tree Planted or Preserved	Square footage deducted from area requiring stormwater treatment
Evergreen: new planting	200 sq. ft. per tree
Deciduous: new planting	100 sq. ft. per tree
Preserve existing trees (either evergreen or deciduous)	Square footage beneath canopy

Self-Retaining Area

- Pervious area that retains first 1" of rainfall on itself and runoff from adjacent impervious area, up to a 2:1 ratio (impervious:pervious)
 - Roof runoff dispersion to depressed landscaped area
 - Partial green roofs
 - Pervious pavement (with adequate storage)
- No special soils required
- Area must be able to retain up to 3" of ponding

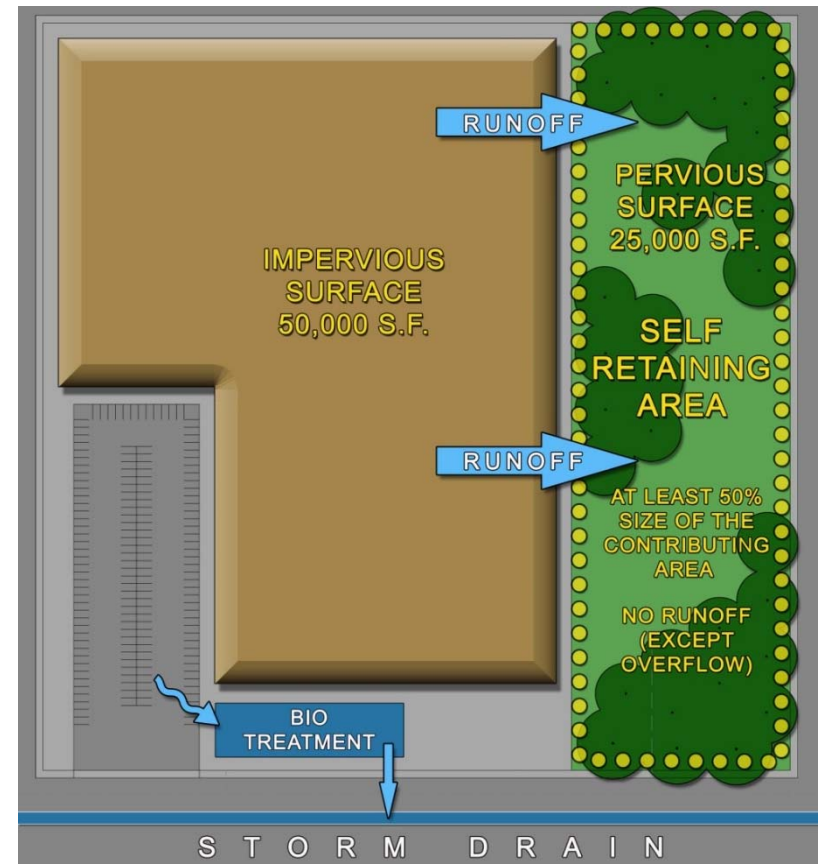


Design of Self-Retaining Areas

- Landscaped areas
 - Plan sheet should indicate a relatively flat, concave, landscaped surface with ponding depth as follows:
Depth = 1 inch + [(Imperv Area ÷ Perv Area) X 1 inch]
 - Elevation of any area drains should be set at top of ponding depth
- Partial green roofs and pervious pavement
 - Calculate depth of water quality volume using equation above
 - Determine depth of media/aggregate require to store the water quality volume

Self-Retaining Areas Reduce the Area that Requires Treatment

- Runoff from **impervious** portions of the project can flow directly to a **pervious** area that is at least 50% of the size of the contributing area
- Runoff from other impervious areas can flow to a smaller treatment measure



C.3.d Sizing Criteria for Treatment Measures

- Volume-based sizing criteria:
 - URQM Method - use formula and volume capture coefficients in “Urban Runoff Quality Management”, WEF/ASCE MOP No. 23 (1998), pages 175-178
 - CASQA BMP Handbook Method - Determine volume equal to 80% of the annual runoff, using methodology in Appendix D of the CASQA BMP Handbook (2003) using local rainfall data
 - **Additional sizing information was developed for San Mateo County rain gages (see C.3 Technical Guidance Appendix C)**

Treatment Measure Design Criteria Regions for San Mateo County

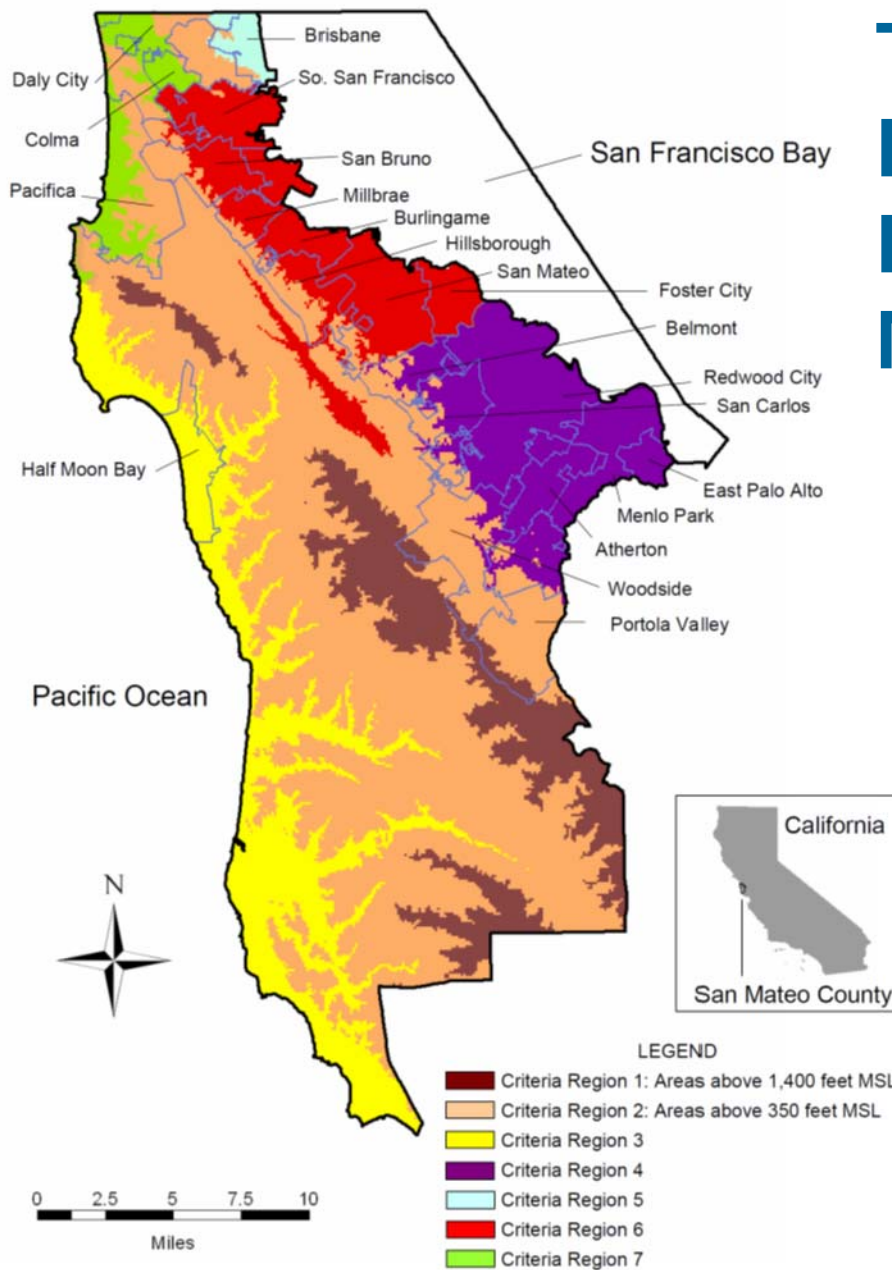


Figure 1 in Appendix 3 of
the C.3 Technical Guidance

C.3.d Sizing Criteria

- Flow-based sizing criteria:
 - Factored Flood Flow - 10% of the 50-year peak flow rate, determined using Intensity-Duration-Frequency curves from local flood control agency
 - **Not generally used**
 - Percentile Rainfall Intensity - Flow of runoff produced by a rain event equal to two times the 85th percentile hourly rainfall intensity
 - **No local data available for San Mateo County**
 - Uniform Intensity - **Flow of runoff resulting from a rain event equal to 0.2 inches per hour intensity**

C.3.d Sizing Criteria

- Flow-based sizing criteria:
 - Simplified Sizing Approach – Variation of Uniform Intensity Method (0.2 in/hr)
 - Surface area of biotreatment measure is sized to be 4% of the contributing impervious area
 - Based on a runoff inflow of 0.2 in/hr (assume equal to the rainfall intensity), with an infiltration rate through the biotreatment soil of 5 in/hr
($0.2 \text{ in/hr} \div 5 \text{ in/hr} = 0.04$)
 - Conservative approach because does not account for surface ponding; but maximizes infiltration

C.3.d Sizing Criteria

- Combination Flow & Volume Design Basis:
 - Treatment systems can be sized to treat “at least 80% of total runoff over the life of the project”
 - Option 1: Use a continuous simulation hydrologic model (typically not done for treatment measures)
 - Option 2: Show how treatment measure sizing meets both flow and volume-based criteria
 - Used for bioretention and flow-through planters
 - **See guidance in Chapter 5, Section 5.1 of C.3 Technical Guidance and Combination Flow-Volume Sizing Worksheet**

Flow- or Volume-Based Sizing for Treatment Measures?

Table 5-1 Flow and Volume Based Treatment Measure Sizing Criteria		
Type of Treatment Measure	LID?	Hydraulic Sizing Criteria
Bioretention area	Yes	Flow- or volume-based or combination
Flow-through planter box	Yes	Flow- or volume-based or combination
Tree well filter	Some	Flow-based
Pervious pavement	Yes	Volume-based
Infiltration trench	Yes	Volume-based
Subsurface infiltration system	Yes	Volume-based
Rainwater harvesting/use	Yes	Volume-based
Media filter	No	Flow-based

Sizing Guidance

- Appendix B of C.3 Technical Guidance
 - Sizing examples
- Appendix C of C.3 Technical Guidance
 - Figure 1: Treatment Measure Design Criteria Regions for San Mateo County
 - Figure 2: Mean Annual Precipitation
- Website:
www.flowstobay.org/newdevelopment
 - Sizing worksheets for determining water quality design volume, and combination flow/volume

Mean Annual Precipitation (inches)

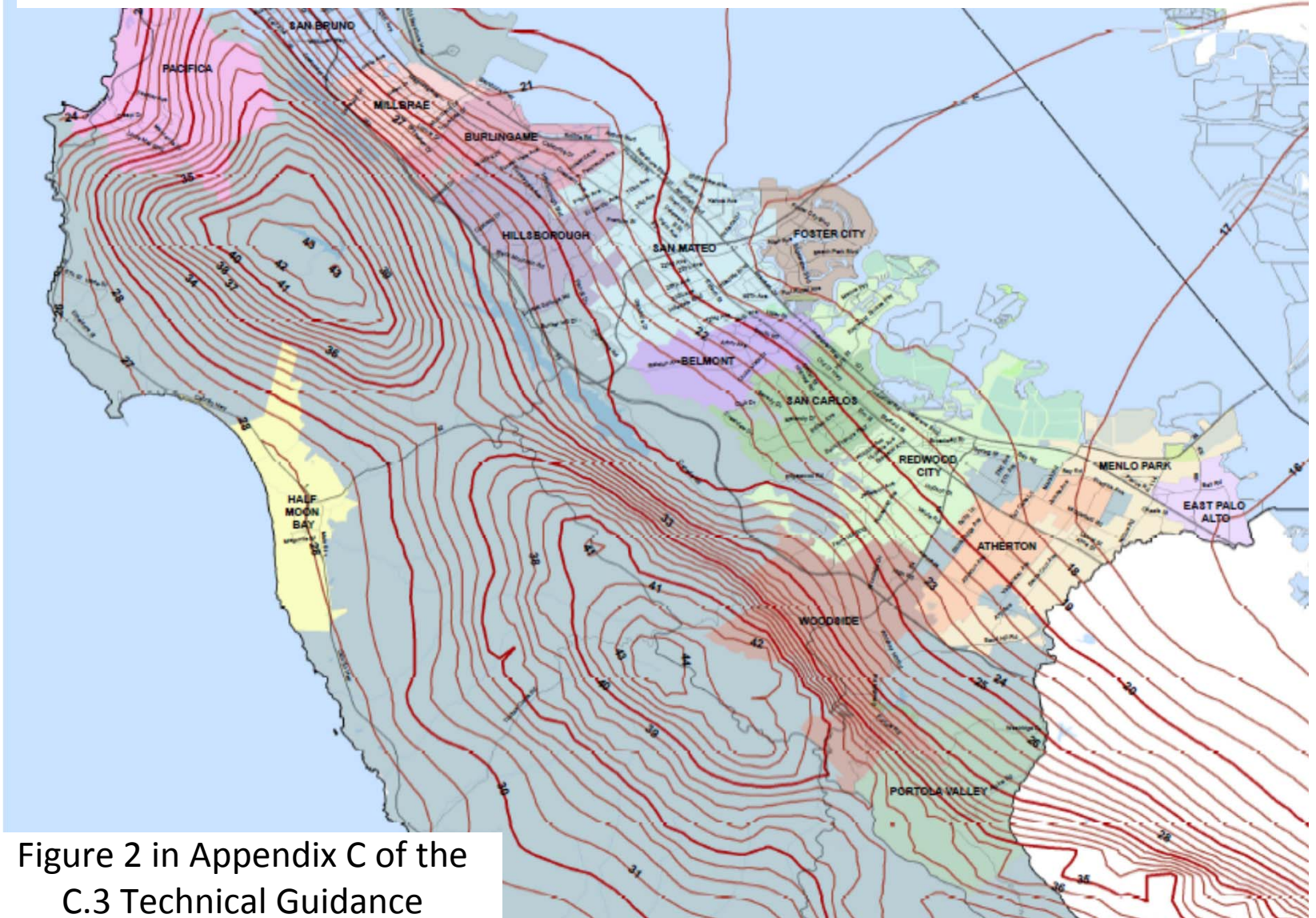
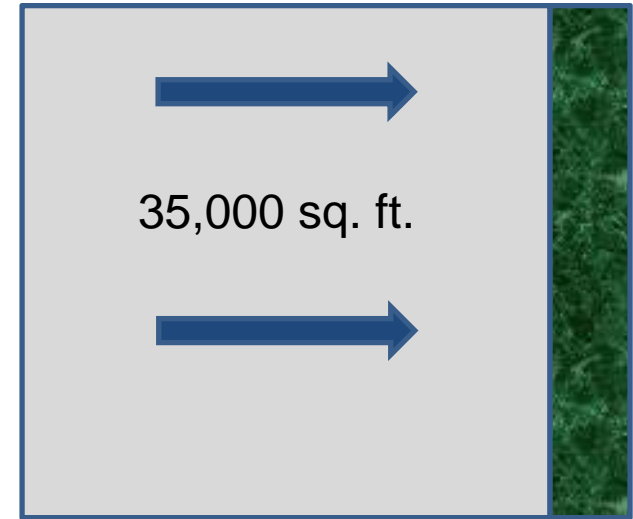


Figure 2 in Appendix C of the
C.3 Technical Guidance

Sizing Example (Volume-based)

- Parking lot in Brisbane
 - Area = 35,000 sq. ft. (0.80 acres)
 - 100% impervious
 - Mean annual precipitation (MAP) = 23"
 - Rainfall Region #5, MAP = 21"
- Use the sizing worksheets to determine the water quality design volume (V_{wQ})
- **Answer: $V_{wQ} = 2,332$ cu. ft.**



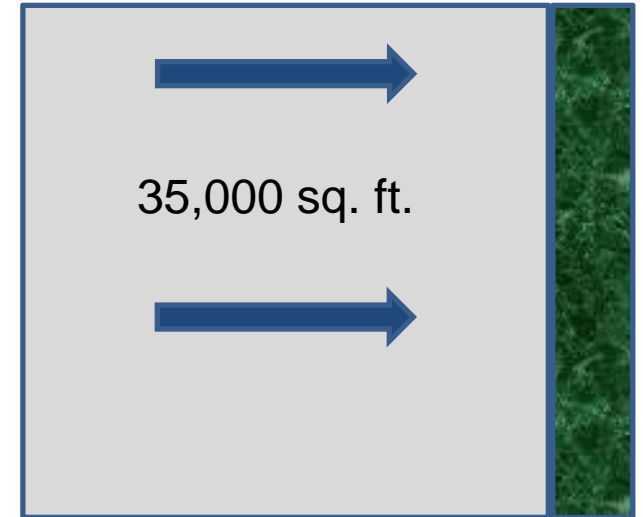
Sizing Bioretention Facilities

- Simplified Sizing (Flow-Based) Approach
 - Surface area is 4% of contributing impervious area
 - Does not consider storage in surface ponding area
- Volume Based Approach
 - Store V_{wQ} in just surface ponding area
 - Store V_{wQ} in ponding area, soil media & drain rock
- Combination Flow and Volume Approach
 - Compute both Q_{wQ} and V_{wQ}
 - “Route” through facility, allowing ponding

Simplified Sizing Example

- Parking lot in Brisbane

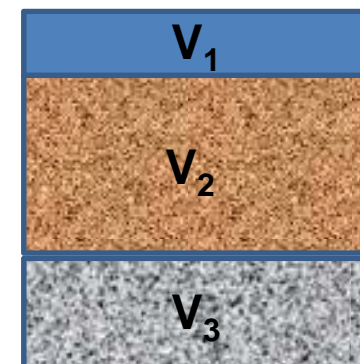
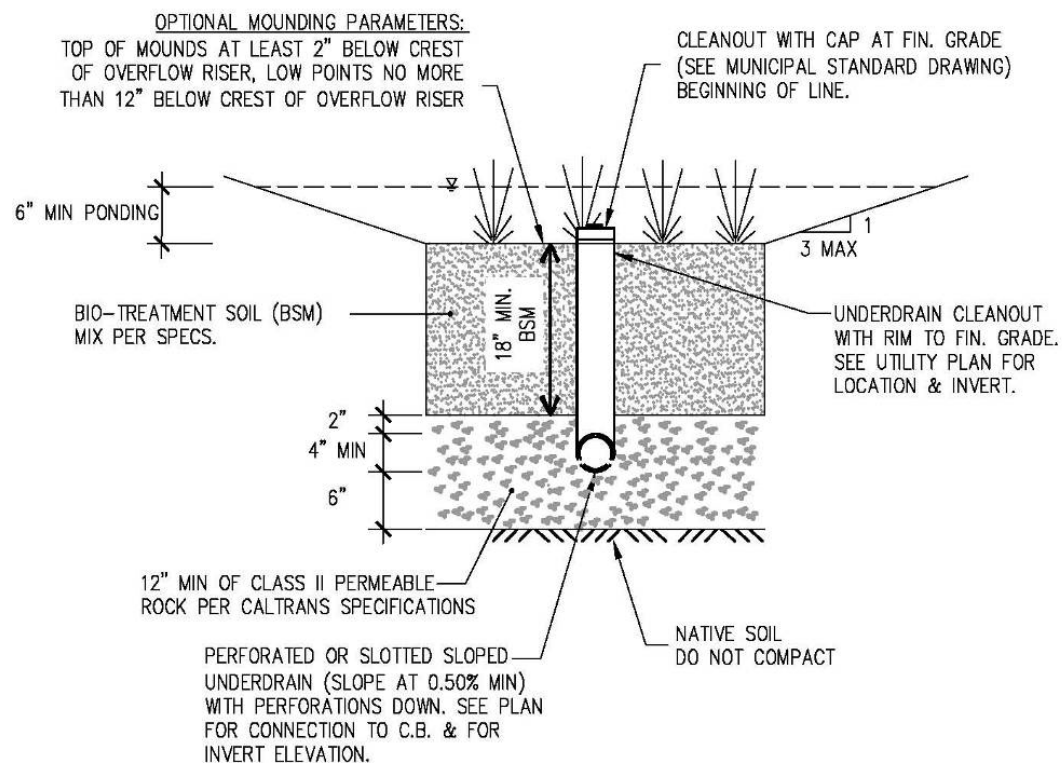
- Area = 35,000 sq. ft. (0.80 acres)
- 100% impervious
- MAP – not needed
- Uniform intensity = 0.2 in/hr



- Surface area of bioretention:


- **Area X 0.04 = 1,400 sq. ft.**
- Note: if drainage area contains pervious area, multiply pervious area by 0.1 and add to impervious area to get “effective impervious area”

Sizing Bioretention Facilities: Volume-Based Approach



Sizing Bioretention Facilities: Volume-Based Approach

Method 1: Store entire volume in surface ponding area



Depth (ft)	Porosity	Volume per sq. ft. (cubic feet)
0.5	1.0	0.5

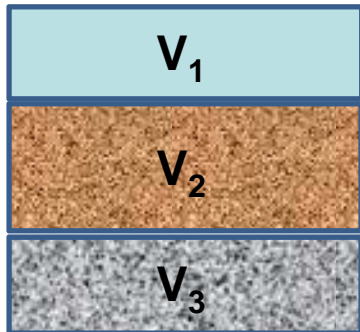
$$\text{Surface Area} = V_{wQ} \text{ (cu.ft.)} \div 0.5 \text{ cu.ft./sq.ft.}$$

Sizing Example:

- **2,332 cu.ft. \div 0.5 cu.ft./sq.ft. = 4,664 sq.ft.**

Sizing Bioretention Facilities: Volume-Based Approach

Method 2: Store volume in ponding area and media



Depth (ft)	Porosity	Volume per sq. ft. (cubic feet)
0.5	1.0	0.5
1.5	0.30	0.45
0.5*	0.40	0.20
Total		1.15

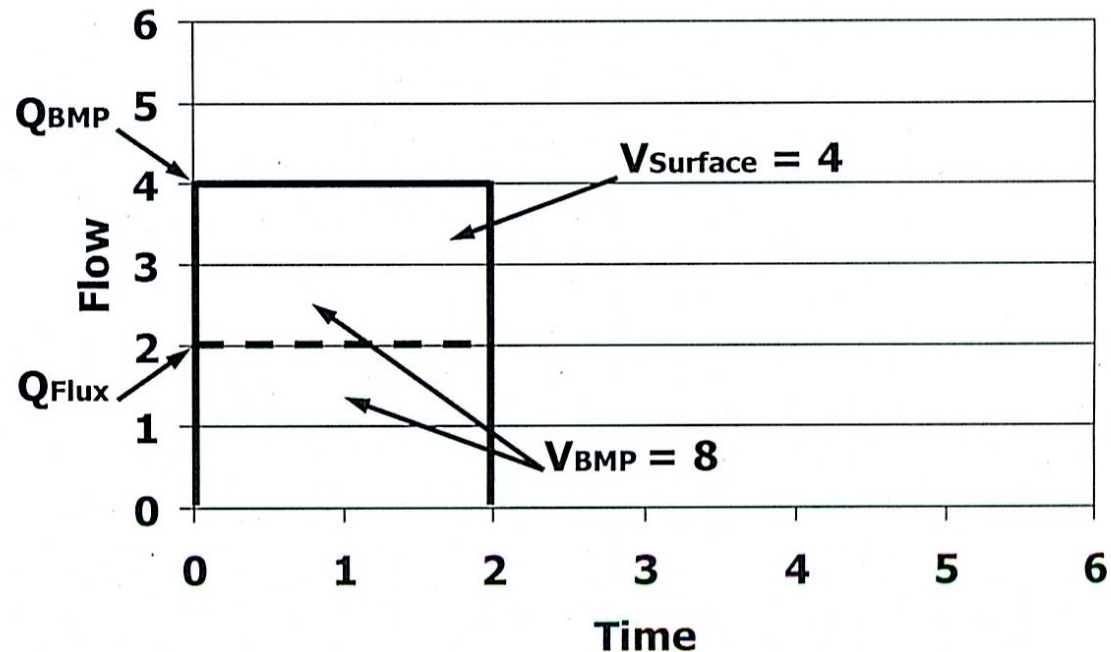
*Depth below underdrain at 6" above bottom

Surface Area = V_{wQ} (cu.ft.) \div 1.15 cu.ft./sq.ft.

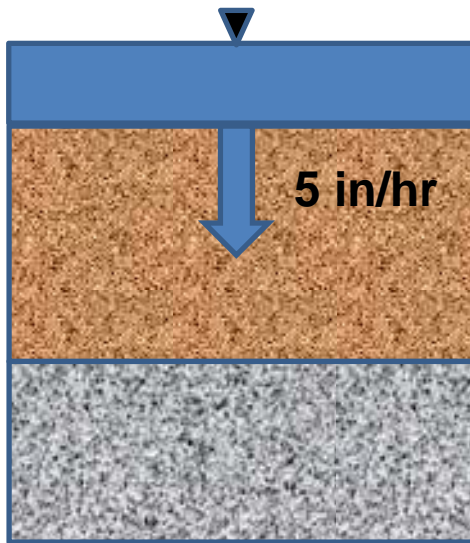
- **2,332 cu.ft. \div 1.15 cu.ft./sq.ft. = 2,028 sq.ft.**

Sizing Bioretention Facilities: Flow & Volume Approach

- “Hydrograph Approach”
 - Runoff is “routed” through the treatment measure
 - Assume rectangular hydrograph that meets both flow and volume criteria

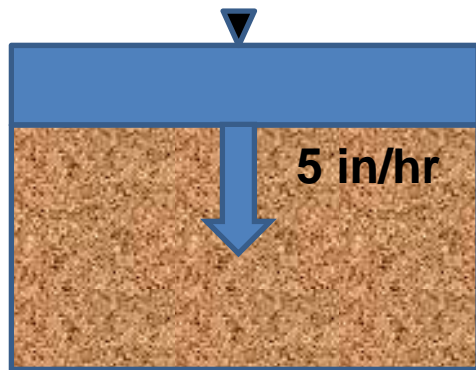


Sizing Bioretention Facilities: Flow & Volume Approach



- Determine VWQ
- Assume constant rainfall intensity of 0.2 in/hr continues throughout the storm (rectangular hydrograph)
- Calculate the duration of the storm by dividing the Unit Basin Storage by the rainfall intensity
- Calculate the volume of runoff that filters through the biotreatment soil at 5 in/hr over the storm duration
- Calculate the volume that remains on the surface and ponding depth

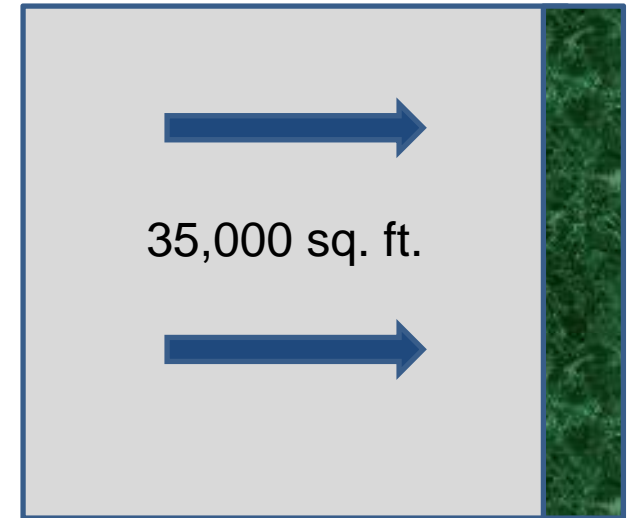
Sizing Bioretention Facilities: Flow & Volume Approach



- To start the calculation, you have to assume a surface area " A_s " -- use 3% of the contributing impervious area as a first guess
- Determine volume of treated water " V_T " during storm:
$$V_T = A_s \times 5 \text{ in/hr} \times \text{duration (hrs)} \times 1 \text{ in/12 ft}$$
- Determine volume remaining on the surface " V_s ":
$$V_s = V_{wQ} - V_T$$
- Determine depth " D " of ponding on the surface:
$$D = V_s \div A_s$$
- Repeat until depth is approximately 6 inches

Sizing Example (Combo Method)

- Parking lot in Brisbane
 - Area = 35,000 sq. ft. (0.80 ac.)
 - 100% impervious
 - $V_{wQ} = 2,332$ cu. ft.
 - Adj. UBS Volume = 0.80 in.
- Use the combination flow and volume sizing worksheet to determine the bioretention surface area
- **Answer: 1,075 sq. ft. (with depth = 6.0")**



Sizing Bioretention Facilities: Comparison of Methods

Example: 35,000 sq. ft. parking lot in Brisbane

MAP= 23 inches, 100% impervious

$V_{BMP} = 2,332$ cu. ft. (80% of annual runoff)

Sizing Method	Surface Area (sq. ft.)
Simplified Method (flow-based)	1,400
Volume ponded on surface	4,664
Volume stored in unit ($V_1+V_2+V_3$)	2,028
Combination flow & volume	1,075



Sizing Pervious Pavement and Infiltration Trenches

- General Principles
 - Store the V_{wQ} in void space of stone base/subbase and infiltrate into subgrade
 - Surface allows water to infiltrate at a high rate
 - Any underdrains must be placed above the void space needed to store and infiltrate the V_{wQ}



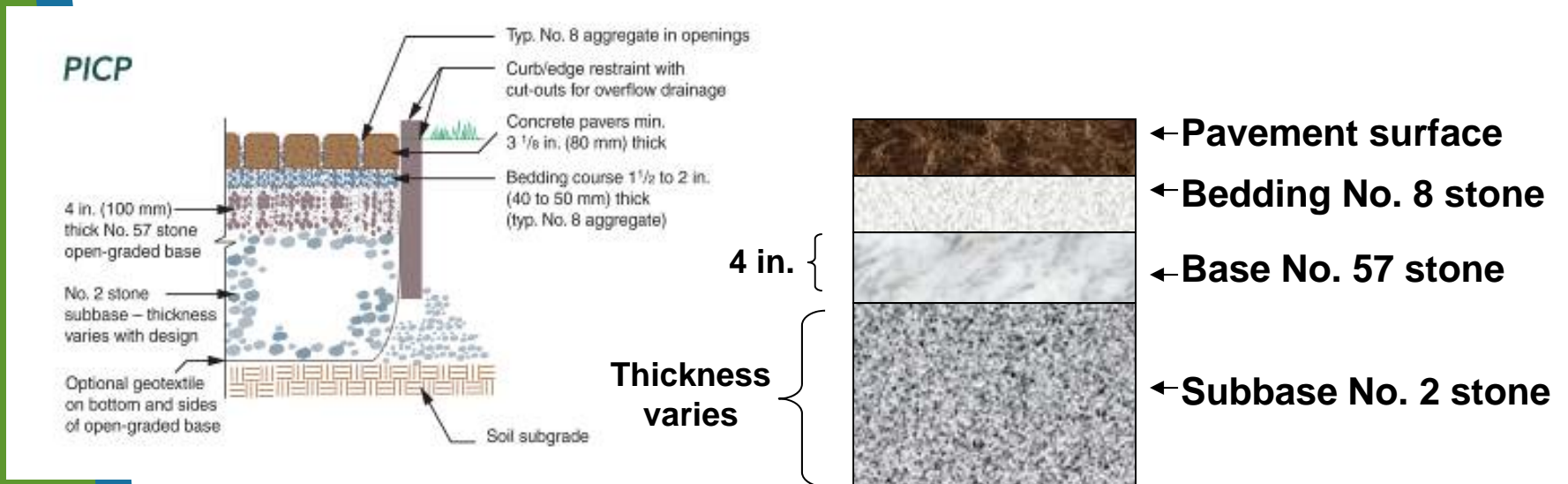
Sizing Pervious Pavement and Infiltration Trenches

■ Pervious Pavement

- May be self-treating area or self-retaining area (accept runoff from other areas)
- Can only be considered a “pervious area” if stone base/subbase sized to store the V_{wQ}
- Can work where native soils have low infiltration rates (stored water depths are relatively small)
- Surface area is usually predetermined
- Base and subbase thickness usually determined by expected traffic load and saturated soil strength
- Slope should be $\leq 3\%$ (or use check dams/trenches in subbase)

Pervious Pavement

Typical Section



- Base and subbase layers available for water storage
- Both typically have 40% void space

Pervious Pavement

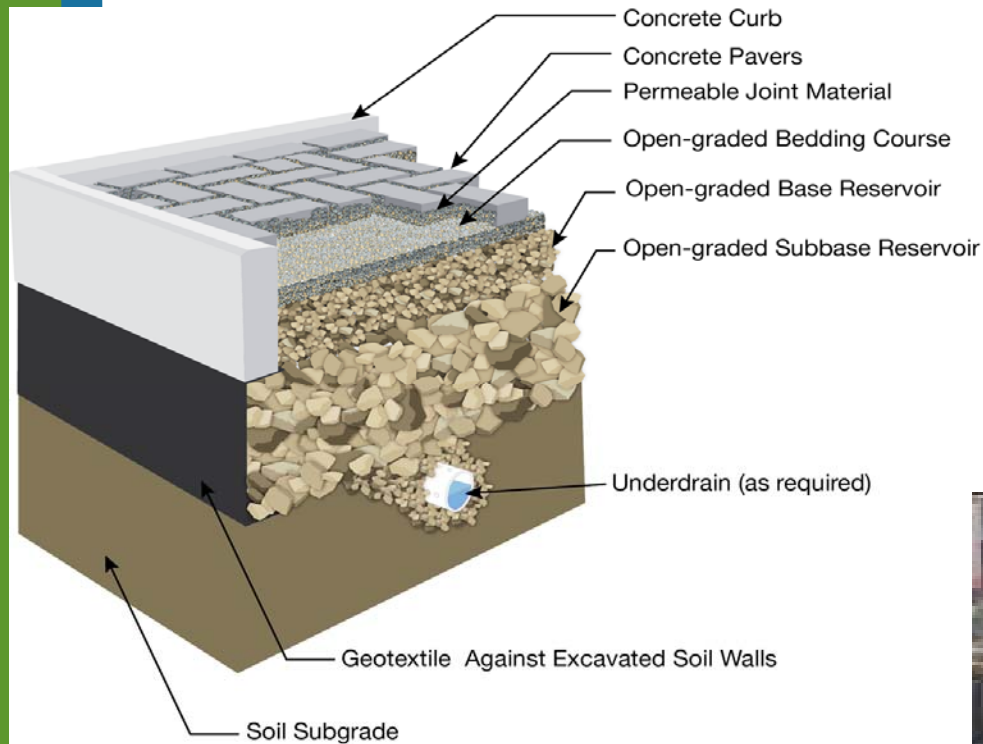
- Approach to Sizing Pervious Pavement
 - Self-Treating
 - Check the depth of the V_{wQ} in base/subbase:
 $UBSV \text{ (in.)} \div 0.40 = \text{Depth (in.)}$

Example: $UBSV = 1.0 \text{ in.}$, $\text{depth} = 2.5 \text{ in.}$
(Minimum depth for vehicular traffic is 10 in.)
 - Check the time required for stored water to drain:
 $UBSV \text{ (in.)} \div \text{Infiltration rate (in/hr)} = \text{Drain time (hrs)}$
(recommend $< 48 \text{ hrs}$)

Pervious Pavement

- Approach to Sizing Pervious Pavement
 - Self-Retaining (receives runoff from adjacent areas)
 - Add the V_{wQ} for adjacent areas to the V_{wQ} for the pervious pavement, divide the total by pervious pavement area
 - Do not exceed 2:1 ratio of contributing area to pervious area
 - Check depth of total V_{wQ} in base/subbase:
 $V_{wQ} \text{ (in.)} \div 0.40 = \text{Depth (in.)}$
Example: $V_{wQ} = 3.0 \text{ in.}$, depth = 7.5 in.
 - Check the time required for stored water to drain:
 $v_{wQ} \text{ (in.)} \div \text{Infiltration rate (in/hr)} = \text{Drain time (hrs)}$

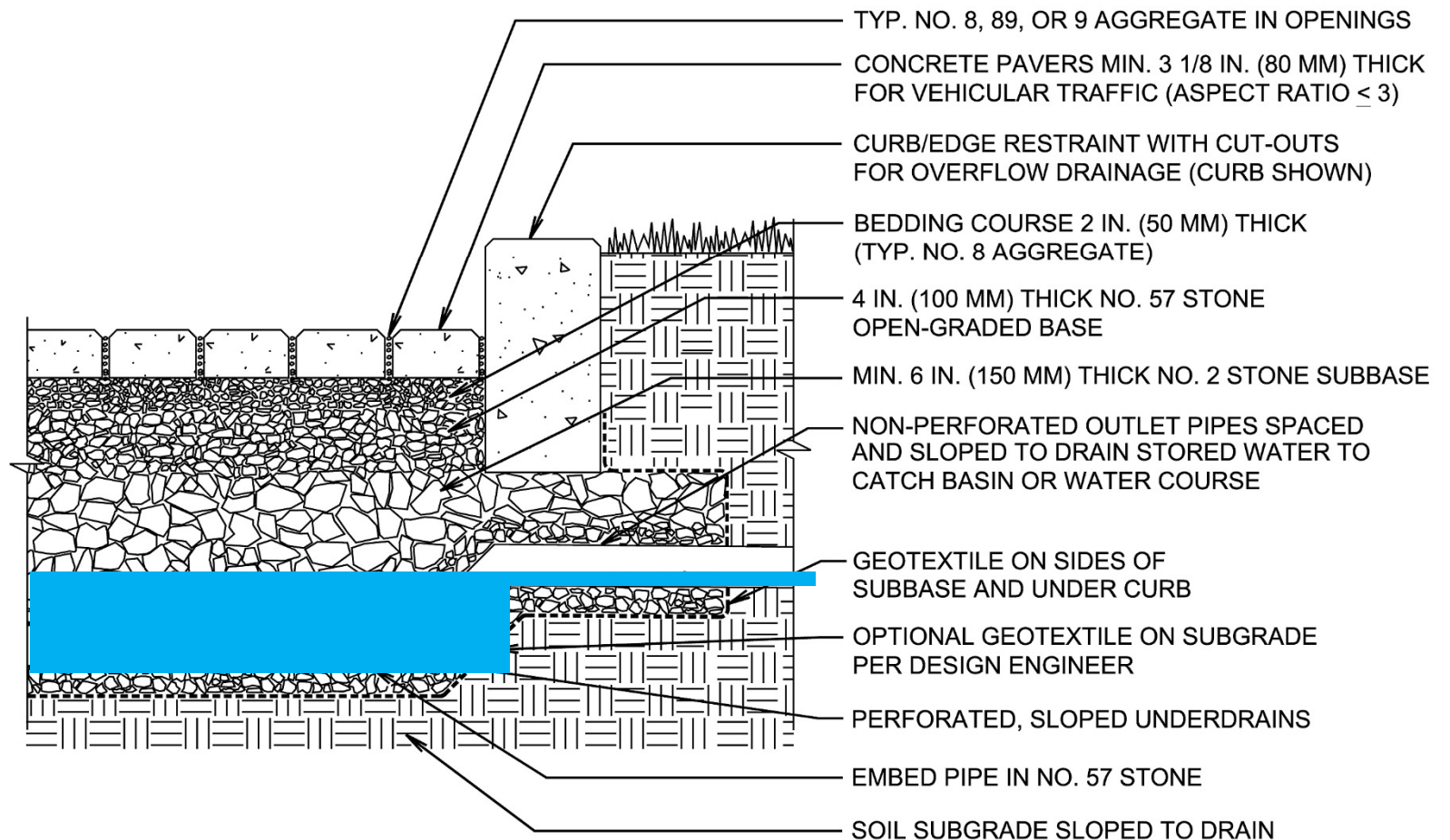
Underdrains: New Approach



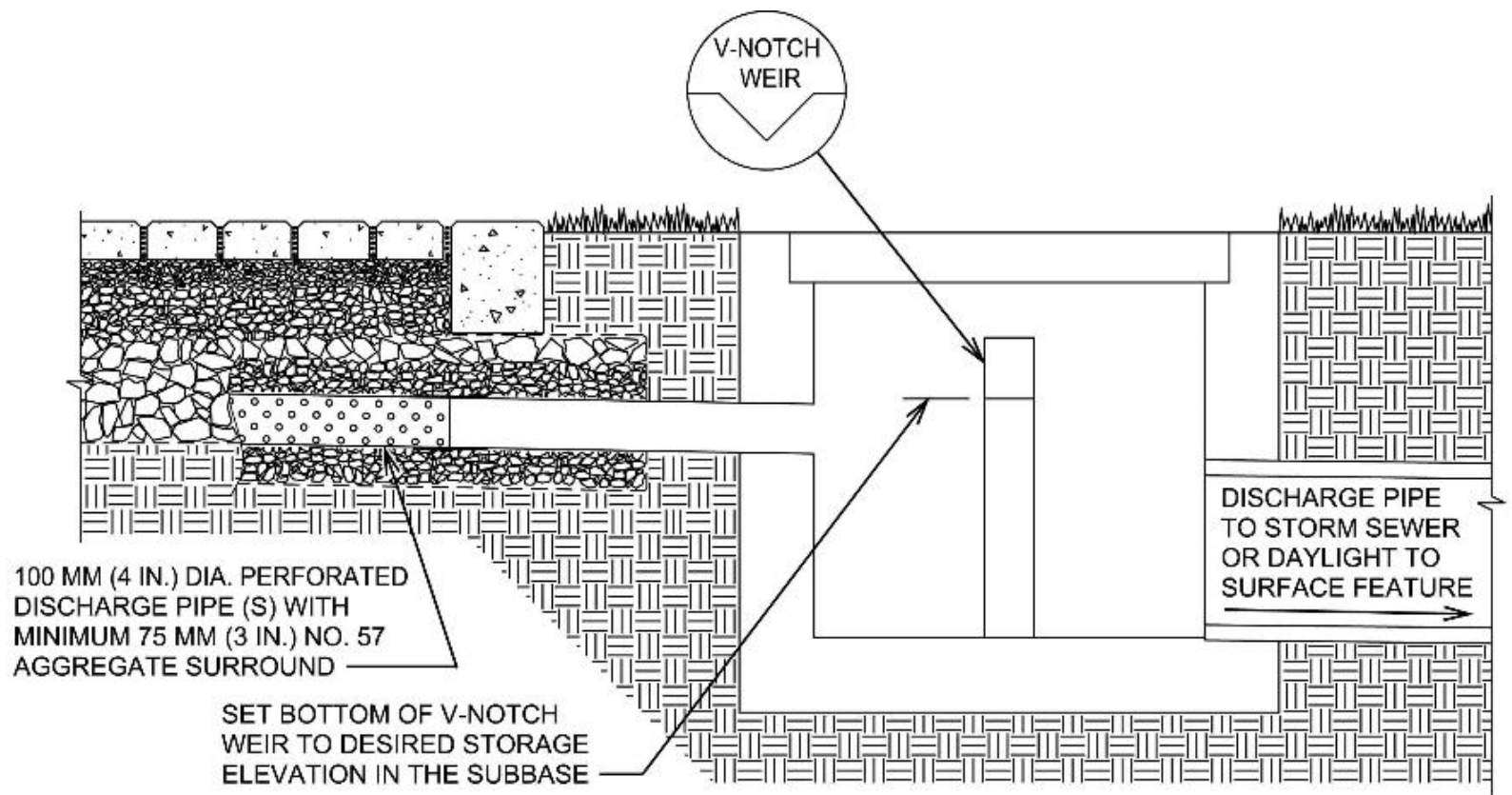
- Underdrain placed in trench at bottom of section with raised outlet to allow water storage in aggregate reservoir



Underdrain: Upturned Elbow



Underdrain: Connection to Catch Basin/Utility Structure



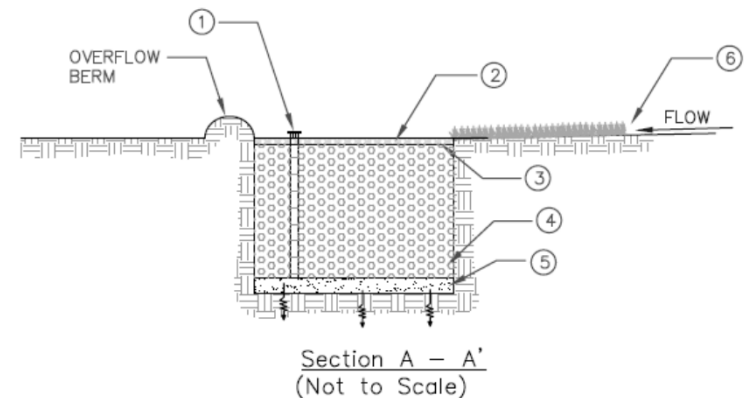
Infiltration Trench Sizing

- Differences from Pervious Pavement
 - More runoff must infiltrate in a smaller footprint
 - Infiltration rate of site soils must be at least 0.5 in/hr (i.e., not suitable for “C” or “D” soils)
 - Trench depths are typically between 3 and 8 feet
 - Infiltration trench is an “infiltration device”
 - Minimum 10-foot separation from seasonal high groundwater level
 - Must meet other MRP requirements
 - Cannot be “deeper than wide” (definition of Class V injection well)

Infiltration Trench Sizing

■ Design Parameters

- Trench depth is calculated based on the soil infiltration rate, aggregate void space, and the trench storage time
- The stone aggregate used in the trench is typically 1.5 to 2.5 inches in diameter, which provides a void space of approximately 35 %
- Trenches should drain within 72 hours
- Place underdrain above void space needed for storage of V_{wq}



Infiltration Trench Sizing

- Approach to Sizing Infiltration Trenches
 - Trench unit storage volume: $S = n \times d$
 n = gravel porosity (0.35); d = gravel depth (ft)
 - Subsoil unit infiltration capacity: $S_i = k \times t \div 12$
 k = subsoil permeability (in/hr); t = time (hrs)
(recommend maximum of 72 hrs)
 - Check for trench drainage by infiltration:
If $S \leq S_i$: Increase depth of media until $S = S_i$
to match trench capacity to infiltration capacity
(may decrease surface area needed)
If $S > S_i$: Decrease depth of media until $S = S_i$
(surface area may increase)

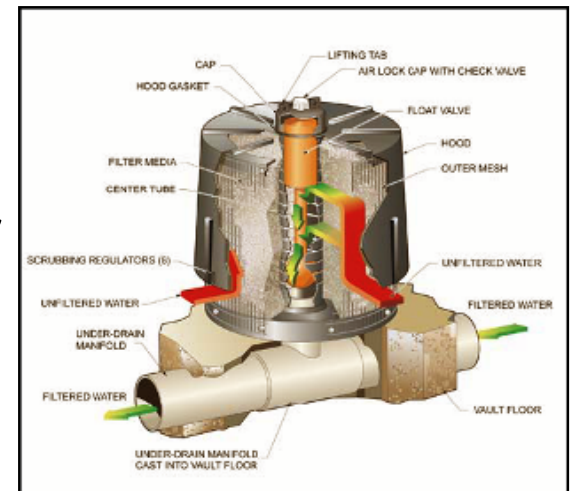
Infiltration Trench Sizing

- Approach to Sizing Infiltration Trenches
 - Determine required trench area:
 - **$A_T = V_{wQ} \div S$**
 A_T = Trench area required to store treatment volume (sq.ft.)
 V_{wQ} = Water quality design volume (cu. ft.)
 S = Trench unit storage volume (cu.ft./sq.ft.)
 - Determine required trench width:
 - **$W = A_T \div L$**
 W = Width of trench (ft.)
 A_T = Required trench area (sq. ft.)
 L = Length of trench (ft.) (normally length of treatment area)

Sizing High-Rate Media Filters

Media Filters (cartridge type)

- Flow-based Treatment Measure
- Determine Q_{wq}
- Select a product that is certified by Washington State TAPE program*
- Determine the TAPE-approved design flow rate per cartridge
- Divide Q_{wq} by the cartridge flow rate to calculate the required number of cartridges (round up)



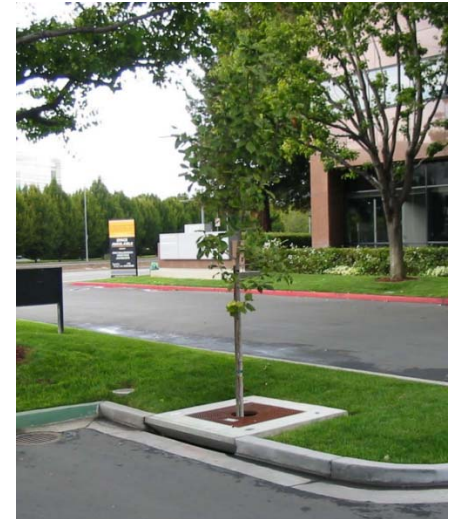
*General Use Level Designation (GULD) for Basic Treatment

<http://www.ecy.wa.gov/programs/wq/stormwater/newtech/technologies.html>

Sizing High-Rate Media Filters

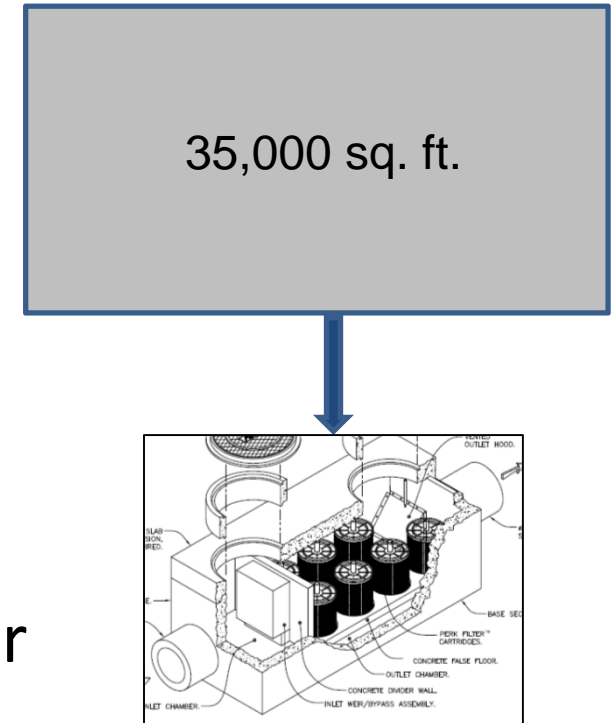
Proprietary Tree Box Filters

- Flow-based Treatment Measure
- Determine Q_{wQ}
- Select a product that is certified by Washington State TAPE program
- Determine the TAPE-approved infiltration rate for the media
- Calculate the required surface area by dividing Q_{wQ} by the infiltration rate (ft/sec)
- A tree box filter that uses biotreatment soil media can be sized like a flow-through planter



Sizing Example (Flow-based)

- Rooftop in Brisbane (Special Project)
 - Area = 35,000 sq. ft. (0.80 ac.)
 - 100% impervious
 - Uniform intensity = 0.2 in/hr
 - Runoff coefficient = 0.90
- Use the Rational Method ($Q = CIA$) determine the water quality design flow, Q_{wQ}
- **Answer: $Q_{wQ} = (0.9)(0.2)(0.8) = 0.144$ cfs**



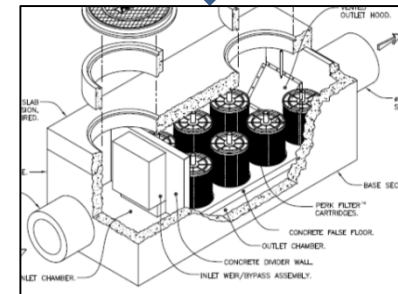
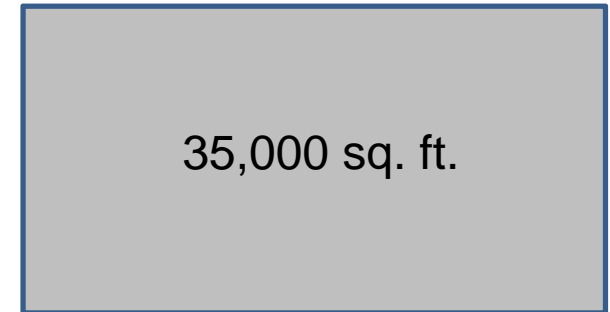
Sizing Example (Flow-based)

- Rooftop in Brisbane
(Special Project)

- $Q_{wQ} = 0.144 \text{ cfs} = 64.6 \text{ gpm}$
- Select media filter type
- Check for TAPE certification and allowable flow rate

- Example: FloGard Perk Filter

- “Size at hydraulic loading rate of $\leq 1.5 \text{ gpm/ft}^2$ of media surface area”
- For 18” cartridge, loading rate is 10.2 gpm/cartridge
- $64.6 \text{ gpm} \div 10.2 \text{ gpm/cartridge} = 6.3$, or 7 cartridges



Questions?



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